COMPARISON OF THE METHOD OF LEAST SQUARES AND THE SIMPLEX METHOD FOR PROCESSING GEODETIC SURVEY RESULTS

POROVNANIE METÓDY NAJMENŠÍCH ŠTVORCOV A SIMPLEXOVEJ METÓDY PRI SPRACOVANÍ VÝSLEDKOV GEODETICKÝCH MERANÍ

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Abstract

The present paper is devoted to the use of the simplex method in the processing of results from geodetic measurements as compared with the standard used method of least squares. Using the simplex method, a minimization problem is usually solved in a standard tabular form by rearranging lines and columns in order to find an optimal solution. The paper points out the simpler, more stable and more efficient way to solve a problem of linear programming through a matrix of relations.

Abstrakt

Predložený príspevok je venovaný použitiu simplexovej metódy pri spracovaní výsledkov geodetických meraní v porovnaní so štandardne používanou metódou najmenších štvorcov (MNŠ). Minimalizačný problém je pri simplexovej metóde zvyčajne riešený tabuľkovou formou, preskupovaním stĺpcov a riadkových operácií, ktorých cieľom je nájdenie optimálneho riešenia. V príspevku je poukázané na numericky nenáročnejšiu, stabilnejšiu a efektívnejšiu cestu riešenia problému lineárneho programovania pomocou maticových vzťahov.

Key words: method of least squares, simplex method, L1 norm, L2 norm, outlier detection

1 INTRODUCTION

In geodetic practice, the least square method (LSM) ranks among standard used processing methods. This method is based on the vector of corrections of the L2 norm which together with the L1 norm is most often applied to the processing of results of geodetic measurements. However, a prerequisite for the proper functioning of the LSM is a normal distribution of errors; otherwise the created probabilistic model is not correct. For this reason, the geodetic practice started to use variant processing methods as well. Such methods include, for example, robust estimation procedures that preserve their function in a certain neighbourhood of a normal distribution, i.e. not fail in case of a moderate failure to comply with this requirement. The more the method is resistant, the more it is robust. Several types of these methods are known, from the robust M-estimates [5],[6],[7],[8] to linear programming methods to which undoubtedly the simplex method belongs. The LSM and the simplex method will be demonstrated on two examples; on the examples of a regression line and a geodetic network. In the first case, the models of a regression line and a geodetic network are loaded by the value of an experimental outlier.

2 METHOD OF LEAST SQUARES

The essence of this method lies in minimizing the sum of squares of deviations in measurements of the behaviour of any quantity or physical phenomenon (3). The least square method is based on the condition of so-called L2-norm; the norm is the number assigned to each n-dimensional vector of residual deviations

 $v = (v_1, v_2, ..., v_n)$ that in some sense characterizes its size [3], [13]. In geodesy, the most commonly used types of objective functions are as follows:

$$\rho(v) = \left(\sum_{i=1}^{n} \left|v_{i}\right|^{p}\right)^{\frac{1}{p}} = \min. \qquad i \in \langle 1, n \rangle$$
(1)

Where:

p - a parameter defining a special type of an objective function,

 v_i – a vector of residual deviations (vector of corrections).

Assuming that p = 2 (L2 norm), the objective function is as follows:

$$\rho(v_{i}) = \left(\sum_{i=1}^{n} |v_{i}|^{2}\right)^{\frac{1}{2}} = \min$$
(2)

which leads to the least square method that leads under certain conditions to the most reliable estimates of unknown quantities, and hence it is the most commonly used method in geodetic practice. The least square method will be explained on a one-dimensional linear model, while all the estimation methods will be demonstrated on the example of a regression line.

Let us assume that the following linear relationship exists between a variable y, variables X (Fig. 1) and a random component u_i :



Where:

y i - dependent variables,

 X_1 , X_2 , ... X_k – independent variables,

 β_0 – the parameter indicating a value of the variable y provided that the variable X is equal to 0, the so-called intercept

 β - the regression coefficient defining the slope of the regression line,

 u_i – the random component.

Provided that we have *n* pairs of the independent variable *X* and the dependent variable *Y* $[x_1, y_1], [x_2, y_2], \dots [x_n, y_n]$ and are sufficiently convinced of the linear dependence, we can construct the line that best describes this fact:

$$\hat{y}_i = b_0 + b_1 x_i$$
 kde i = 1,2···nvyrovnávajúca regresná priamka (4)

Where:

 \hat{y}_i – the adjusted one (theoretical value) of the dependent variable,

x_i - the dependent variable for the i-th observation,

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(3)

b₀ – the point estimate of the parameter $\beta_{0,}$

b - the point estimate of the parameter β .

For residual deviations (in the geodesy vector of corrections) $\mathbf{v} = \mathbf{v}_1, \mathbf{v}_2, \dots, \mathbf{v}_n$, the relation $v_i = y_i - \hat{y}_i$ applies.



x-independent variables

Fig. 2 Residual deviations

Since the residual deviations may take a positive as well as negative value, they null each other. The nullifying problem can be solved just by applying the LSM, the principle of which consists in the sum of squares of deviations and not of the deviations themselves. The LSM formulation is then as follows:

$$\sum_{i=1}^{n} (y_i - \hat{y}_i)^2 = \sum_{i=1}^{n} (y_i - b_0 - b_1 x_i)^2 = \sum_{i=1}^{n} (v_i)^2 = \min.$$
(5)

The minimum of a function of two variables can be found by placing its partial derivatives under both variables (coefficients $b_0 a b_1$) equal to zero:

$$\frac{\partial \left(\sum_{i=1}^{n} (y_i - b_0 - b_1 x_i)^2\right)}{\partial (b_1)} = 2 \left(\sum_{i=1}^{n} (y_i - b_0 - b_1 x_i)(-x_i)\right) = 0,$$

$$\frac{\partial \left(\sum_{i=1}^{n} (y_i - b_0 - b_1 x_i)^2\right)}{\partial (b_0)} = 2 \left(\sum_{i=1}^{n} (y_i - b_0 - b_1 x_i)(-1)\right) = 0.$$
(6)

By an appropriate algebraic transformation, we get a system of normal equations of two variables b_0 and $b_{l,}\!\!:$

$$\sum_{i=1}^{n} y_i = nb_0 + b_1 \sum_{i=1}^{n} x_i,$$

$$\sum_{i=1}^{n} y_i x_i = b_0 \sum_{i=1}^{n} x_i + b_1 \sum_{i=1}^{n} x_i^2.$$
(7)

For the variables b_0 and b_1 it is true:

1

$$b_{0} = \frac{\sum_{i=1}^{n} y_{i} \sum_{i=1}^{n} x_{i}^{2} - \sum_{i=1}^{n} x_{i} y_{i} \sum_{i=1}^{n} x_{i}}{n \sum_{i=1}^{n} x_{i}^{2} - (\sum_{i=1}^{n} x_{i})^{2}} \quad b_{1} = \frac{n \sum_{i=1}^{n} x_{i} y_{i} - \sum_{i=1}^{n} x_{i} \sum_{i=1}^{n} y_{i}}{n \sum_{i=1}^{n} x_{i}^{2} - (\sum_{i=1}^{n} x_{i})^{2}}$$
(8)

In the processing of geodetic measurements, methods of adjustment are used, by which the most probable value of a quantity and its accuracy characteristics are determined. In determining the parameters of the regression line, a mediating adjustment was used [1],[10],[9],[12], where the relationship between the measured and unknown variables is expressed by the intermediating function of the searched unknown parameters, the so-called estimates:

$$y_i = l_i + v_i = f(\widehat{\theta_1}, \widehat{\theta}_2, \cdots \widehat{\theta}_k)$$
(9)

For the vector of residues, the following is valid:

$$\mathbf{v} = \mathbf{A}\hat{\boldsymbol{\Theta}} - \mathbf{I} \,, \tag{10}$$

leading to the Guss-Markov model:

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$$\mathbf{v} = \mathbf{A}\hat{\Theta} - \mathbf{I}$$
(11)
$$\sum_{l} = \sigma_0^2 \mathbf{Q}_l$$

It applies for LSM:

$$\rho = \sum_{i=1}^{n} (p_i v_i)^2 = \sum_{i=1}^{n} v^T \mathbf{P} v = \min ., \quad \mathbf{P}...... \text{ matica váhovýchkoeficientov}.$$
(12)

matrix of weight coefficients

The minimum of the function of two variables can be found again by placing the partial derivatives of the function (12) equal to zero $\frac{\partial(v^T \mathbf{P}v)}{\partial \hat{\Theta}}$ and so getting the system of two equations, from which the searched variables (estimates) can be determined:

$$(\mathbf{A}^{T}\mathbf{P}\mathbf{A}\hat{\Theta}) - (\mathbf{A}^{T}\mathbf{P}\mathbf{I}) = 0 \rightarrow (\mathbf{N}\hat{\Theta}) - (\mathbf{A}^{T}\mathbf{P}\mathbf{I}) = 0, \mathbf{N} - \text{matica koeficientov normálnychrovníc}$$
(13)

matrix of coefficients of normal equations

When using the LSM for searching the estimates, (indices), the following applies after adjusting the parameters:

$$\hat{\boldsymbol{\Theta}} = (\mathbf{A}^T \mathbf{P} \mathbf{A})^{-1} (\mathbf{A}^T \mathbf{P} \mathbf{I}) = \mathbf{N}^{-1} (\mathbf{A}^T \mathbf{P} \mathbf{I}) .$$
(14)

3 SIMPLEX METHOD

The simplex method is an iterative computational procedure that is used to find optimal solutions whereas the objective (minimized) function must be in a canonical form. The minimization problem is typically solved by means of methods of linear programming in a tabular form by rearranging columns and line unless the objective function optimization is reached. The paper presents a simpler procedure of processing based on the principle of matrix solution [2],[4]. This method will again be presented on the example of a regression line. The model line was chosen not only because of its simplicity, but especially considering the fact that in the processing of results from geodetic measurements there is a need, quite often, to determine the accuracy of the measured length which can be expressed by the following relationship:

$$STD = appm + b \dots modified form of regression line$$
 (15)

Where:

STD - the standard deviation of a measured length,

- a the parameter reflecting the impact of errors dependent on the measured length which take into account the influence of the physical environment,
- b the parameter reflecting the impact of the errors independent of the measured length.

The functional relationship for the correction of the intermediate variable can be expressed as follows:

$$\nu_{(n,1)} = \mathbf{A} \cdot \hat{\mathbf{O}}_{\mathbf{k},\mathbf{l}} - \mathbf{f}_{\mathbf{n},1}, \qquad (16)$$

where **v** is the vector of corrections, **A** the matrix of coefficients, $\hat{\Theta}$ is the vector of unknown parameters (estimates), **f** is the vector of observations, *n* is a number of measurements, and *k* is a number of necessary measurements (determined parameters). Within the search of an optimal solution, first the measured values **f** are divided to the **basic variables** (needed measurements) and **non-basic variables** (r = n-k, r-a number of redundant measurements).

$$\begin{bmatrix} \mathbf{v}_{(1)} \\ \mathbf{v}_{(2)} \end{bmatrix} + \begin{bmatrix} \mathbf{A}_{(1)} \\ \mathbf{A}_{(2)} \end{bmatrix} \hat{\boldsymbol{\Theta}} = \begin{bmatrix} \mathbf{f}_{(1)} \\ \mathbf{f}_{(2)} \end{bmatrix}.$$
(17)

The vector f can be broken down as follows:

$$\begin{bmatrix} \mathbf{f}_{(1)} \\ \mathbf{f}_{(2)} \end{bmatrix} = \begin{bmatrix} \mathbf{l}_{(1)}^{*} \\ \mathbf{l}_{(2)}^{*} \end{bmatrix} - \begin{bmatrix} \mathbf{l}_{(1)} \\ \mathbf{l}_{(2)} \end{bmatrix}.$$
 (18)

In the case of observations with varying accuracy, the L1 norm can be derived from the objective function L2 norm:

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$$\rho_{L2} = \mathbf{v}^T \mathbf{P} \mathbf{v} = \sum_{i=1}^n \mathbf{v}_i^2 p_{ii} = \sum_{i=1}^n (\mathbf{v}_i \sqrt{p_{ii}}) (\mathbf{v}_i \sqrt{p_{ii}}) = \min.$$
(19)

To determine the individual elements of the weighting matrix **P**, the Cholesky decomposition is used:

$$\mathbf{P} = \mathbf{S} \cdot \mathbf{S}^T \,. \tag{20}$$

For the functional relationships of measured values, it applies:

$$\mathbf{SA}\mathbf{\hat{\Theta}} \approx \mathbf{S.f}$$
 . (21)

The relationship for the objective function for the L1 norm after the adjustment takes the form:

$$\rho_{L1} = (vecd(\mathbf{S})^T | \mathbf{v} | \to \min.$$
(22)

Where *vectd* () is a column vector and the matrix $|\mathbf{v}|$ has the following form:

$$|v| = ||v||_{t=1}^{h}$$
 with elements on diagonal. (23)

For calculating the unknown parameters, it applies:

$$\Theta = (\mathbf{S}_{(1)} \cdot \mathbf{A}_{(1)})^{-1} \mathbf{S}_{(1)} \mathbf{f}_{(1)} = \mathbf{A}_{(1)} \mathbf{f}_{(1)}, \qquad (24)$$

Where $\mathbf{S}_{(1)}$ is the diagonal matrix with dimensions $(k \ge k)$.

The vector of corrections is divided into two parts:

$$\mathbf{v}_{(1)} = \mathbf{f}_{(1)} - \mathbf{A}_{(1)} \dot{\mathbf{\Theta}} = \mathbf{f}_{(1)} - \mathbf{A}_{(1)} \mathbf{A}_{(1)}^{-1} \mathbf{f}_{(1)} = \mathbf{0}, \ \dots \ \text{residues are always equal } \mathbf{0}$$
(25)

For the non-zero vector of corrections $\mathbf{v}_{(2)}$, it applies:

$$\mathbf{v}_{(2)} = \mathbf{f}_{(2)} - \mathbf{A}_{(2)} \mathbf{\Theta} = \mathbf{f}_{(2)} - \mathbf{A}_{(2)} \mathbf{A}_{(1)}^{-1} \mathbf{f}_{(1)} \,.$$
(26)

The assignment is solved by linear programming methods and is defined as follows:

$$r^{T}\mathbf{x} = \rho_{L1},$$

$$\mathbf{A}\mathbf{x} = g.$$
(27)

where $\mathbf{x} \ge 0$, \mathbf{r} is a minimizing funkcion also called *"cost vector"* of size of $t \ge 1$, t=2k+2n, x is the vector of non-negative variables, \mathbf{A} is the matrix of coefficients.

In order to meet the condition $x \ge 0$ for linear programming, the calculated estimates and the vector of corrections is divided into two non-negative components:

$$\begin{split} \widetilde{\mathbf{\Theta}} &= \delta - \gamma \quad \delta, \gamma \ge 0, \\ \mathbf{v} &= \mathbf{y} - r \quad \mathbf{y}, \ r \ge 0 \end{split}$$

$$\begin{aligned} \mathbf{v}_{\mathbf{i}} &= y_i \ \text{ak} \ v_i > 0, r_i = 0, -\mathbf{v}_{\mathbf{i}} = r_i \ \text{ak} \ v_i < 0, y_i = 0, \\ \mathbf{v}_i &= 0 \ \text{ak} \ y_i = 0, r_i = 0, |v_i| = y_i + r_i \end{aligned}$$

$$(28)$$

The target function (27) can be written in a matrix form as well:

$$\begin{bmatrix} \mathbf{S}_{(1)}\mathbf{A}_{(1)} & -\mathbf{S}_{(1)}\mathbf{A}_{(1)} & \mathbf{I} & \mathbf{0} & -\mathbf{I} & \mathbf{0} \\ \mathbf{S}_{(2)}\mathbf{A}_{(2)} & -\mathbf{S}_{(2)}\mathbf{A}_{(2)} & \mathbf{0} & \mathbf{I} & \mathbf{0} & -\mathbf{I} \end{bmatrix} \begin{bmatrix} \boldsymbol{\delta} \\ \boldsymbol{\gamma} \\ \mathbf{S}_{(1)}\boldsymbol{y}_{(1)} \\ \mathbf{S}_{(2)}\boldsymbol{y}_{(2)} \\ \mathbf{S}_{(1)}\boldsymbol{z}_{(1)} \\ \mathbf{S}_{(2)}\boldsymbol{z}_{(2)} \end{bmatrix} = \begin{bmatrix} \mathbf{S}_{(1)}f_{(1)} \\ \mathbf{S}_{(2)}f_{(2)} \end{bmatrix}$$

$$\begin{bmatrix} \mathbf{0}^{T} & \mathbf{0}^{T} & \mathbf{1}^{T} & \mathbf{1}^{T} & \mathbf{1}^{T} & \mathbf{1}^{T} \end{bmatrix} \begin{bmatrix} \boldsymbol{\delta} \\ \boldsymbol{\gamma} \\ \mathbf{S}_{(1)}\boldsymbol{y}_{(1)} \\ \mathbf{S}_{(2)}\boldsymbol{y}_{(2)} \\ \mathbf{S}_{(2)}\boldsymbol{y}_{(2)} \\ \mathbf{S}_{(1)}\boldsymbol{z}_{(1)} \\ \mathbf{S}_{(2)}\boldsymbol{z}_{(2)} \end{bmatrix} = \phi_{L1}$$

$$(29)$$

where **1** is the all-ones vector $(1^T = [1, 1 \dots 1])$. The default simplex table has the following structure:

The calculation can be tabulated using the rearrangement operations of columns and lines; in the paper, the procedure of transforming the simplex table using a "*pivot*" matrix K was used:

$$\mathbf{K} = \begin{bmatrix} I & 0^{T} & \mathbf{1}^{T} \\ \mathbf{0} & \mathbf{S}_{(1)}\mathbf{A}_{(1)} & \mathbf{0} \\ \mathbf{0} & \mathbf{S}_{(2)}\mathbf{A}_{(2)} & \mathbf{I}^{*} \end{bmatrix}, T^{*} = \mathbf{K}^{-1}.T, \qquad (31)$$

 \mathbf{I}^* - the variable pivot matrix; its diagonal elements have a value +1, or -1 to meet the condition met $v_{(2)} \ge 0$.

The iterative process is carried out by multiplying the initial simplex table (30) with the pivot matrix in inverse form. The structure of the transformed simplex table after the adjustment is as follows:

Where the elements of the table can be determined according to the following relationship:

$$\mathbf{u}_{1} = \mathbf{I}^{T} \mathbf{I} \mathbf{S}_{(2)} \mathbf{A}_{(2)} \mathbf{A}_{(1)}^{-1} f_{(1)} - \mathbf{I}^{T} \mathbf{I}^{*} \mathbf{S}_{(2)} f_{(2)},$$

$$\mathbf{U}_{2} = \mathbf{I}^{*} \mathbf{S}_{2} \mathbf{A}_{(2)} (\mathbf{S}_{(1)} \mathbf{A}_{(1)})^{-1},$$

$$\mathbf{u}_{3} = -\mathbf{I}^{*} \mathbf{S}_{(2)} \mathbf{A}_{(2)} \mathbf{A}_{(1)}^{-1} f_{(1)} + \mathbf{I}^{*} \mathbf{S}_{(2)} f_{(2)}.$$
(33)

The objective function is transformed to minimize the function **r** so-called reduced *"cost"* vector (34) which can be simplified in comparison with the original form, because its elements $\mathbf{1}^T - \mathbf{1}^T \mathbf{I}^*$ and $\mathbf{1}^T - \mathbf{1}^T \mathbf{U}_2$ have a value of 0 or 2.

$$\mathbf{r}^{T} = \left[\mathbf{0}^{T}, \mathbf{0}^{T}, \mathbf{1}^{T} + \mathbf{1}^{T}\mathbf{U}_{2}, \mathbf{1}^{T} - \mathbf{1}^{T}\mathbf{I}^{*}, \mathbf{1}^{T} - \mathbf{1}^{T}\mathbf{U}_{2}, \mathbf{1}^{T} + \mathbf{1}^{T}\mathbf{I}^{*}\right]$$
(34)

The optimal solution is found when for all the elements of the vector $\mathbf{r} \ge 0$ applies which can be transformed to the following condition:

$$-1^{T} \le \mathbf{S}_{(2)} \mathbf{A}_{(2)} \mathbf{S}_{(1)} \mathbf{A}_{(1)} \le 1^{T}$$
(35)

4 EMPIRICAL DEMONSTRATION

In the following chapter, the method of least squares and the simplex method are presented on the example of a regression line and a geodetic network. The regression line and the geodetic network in the first example are loaded by a normal distribution of measurements; due to the investigation of the properties of the used estimation methods, the line and the geodetic network are loaded by an experimental outlier before the adjustment.

4.1 Application of estimation methods using the example of a regression line loaded by a normal distribution of measurements

The regression line in this case is expressed by the equation of a rangefinder as follows:

$$STD = \sigma_D = 2.ppmd + 3[mm] \cdots rovnica regresnejpriamky,$$
 (36)

Where:

STD - a standard deviation of the measured length,

d - the measured length,

ppm - parts per million (10^{-6}) .

The deterministic model of the regression line loaded by a normal distribution of measurements is presented in a graphic form in **Chyba! Nenalezen zdroj odkazů**. The experimental values shown in blue are simulated in the MATLAB environment by the function *normrnd (normal random numbers)*.



Fig. 3 Deterministic model of the regression line loaded by normal distribution

Tab. 1 Final simplex table for the regression line loaded by a normal distribution of measurements

а	1	0	-1	0	-0.625	0.625	0	0	0	0	0	0	0	0	0.625	-0.625	0	0	0	0	0	0	0	0	1.9375
b	0	1	0	-1	1.125	-0.125	0	0	0	0	0	0	0	0	-1.125	0.125	0	0	0	0	0	0	0	0	2.8125
v ₂	0	0	0	0	0.875	0.125	-1	0	0	0	0	0	0	0	-0.875	-0.125	1	0	0	0	0	0	0	0	0.1875
V ₃	0	0	0	0	-0.75	-0.25	0	1	0	0	0	0	0	0	0.75	0.250	0	-1	0	0	0	0	0	0	-0.5250
v4	0	0	0	0	-0.625	-0.375	0	0	1	0	0	0	0	0	0.625	0.375	0	0	-1	0	0	0	0	0	-0.9375
v ₅	0	0	0	0	0.500	0.500	0	0	0	-1	0	0	0	0	-0.500	-0.500	0	0	0	1	0	0	0	0	0.0500
v ₆	0	0	0	0	0.375	0.625	0	0	0	0	-1	0	0	0	-0.375	-0.625	0	0	0	0	1	0	0	0	0.1375
v ₇	0	0	0	0	-0.25	-0.750	0	0	0	0	0	1	0	0	0.250	0.750	0	0	0	0	0	-1	0	0	-0.1750
v ₈	0	0	0	0	-0.125	-0.875	0	0	0	0	0	0	1	0	0.125	0.875	0	0	0	0	0	0	-1	0	-0.5875
v ₁₀	0	0	0	0	-0.125	1.125	0	0	0	0	0	0	0	-1	0.125	-1.125	0	0	0	0	0	0	0	1	0.2875
r	0	0	0	0	1.125	0.875	2	0	0	2	2	0	0	2	0.875	1.125	0	2	2	0	0	2	2	0	-2.8875

 Tab. 2 Results of adjustment of the regression line loaded by a normal distribution of the LSM and L1 norm

	Th	eoretical line			LSM		L1
M. No.	d	σ_{d}	$\sigma_{d}_{_{_{_{_{_{_{det}m.}}}}}}$	3	v	Δ	v
	[mm]	[mm]	[mm]	[mm]	[mm]	[mm]	[mm]
1.	200.00	3.20	3.40	0.20	0.2473	-0.05	0.0000
2.	400.00	3.40	3.80	0.40	0.4145	-0.01	-0.1875
3.	600.00	4.50	4.20	-0.30	-0.3182	0.02	0.5250
4.	800.00	5.30	4.60	-0.70	-0.7509	0.05	0.9375
5.	1000.00	4.70	5.00	0.30	0.2164	0.08	-0.0500
6.	1200.00	5.00	5.40	0.40	0.2836	0.12	-0.1375
7.	1400.00	5.70	5.80	0.10	-0.0491	0.15	0.1750
8.	1600.00	6.50	6.20	-0.30	-0.4818	0.18	0.5875
9.	1800.00	6.30	6.60	0.30	0.0855	0.21	0.0000
10.	2000.00	6.40	7.00	0.60	0.3527	0.25	-0.2875

$$\varepsilon = \sigma_d - \sigma_d$$
, $\Delta = \varepsilon - v$
determ. exp.

Parameters of regression line:

Deterministic form of line Parameters of line estimated by LSM Parameters of line estimated by Hampel method y= 3.0[mm]+ 2.0*ppm*d y= 3.1[mm]+ 1.8*ppm*d y= 2.8[mm]+ 1.9*ppm*d



Fig. 4 Graphical interpretation of LSM and L1 norms on the example of the regression line loaded by normal distribution

From the results of the adjustment (Tab. 2) of such proposed regression line, it is evident that in case of the load of the line by normal distribution of measurements neither the LSM, nor the simplex method in the set of measured data revealed any outlier ("which go beyond") value and the results of the adjustment are very similar. The graph of the regression line behaviour after the adjustment by individual estimate procedures is presented in **Chyba! Nenalezen zdroj odkazů.**

4.2 Application of estimation methods using the example of the regression line loaded by an experimental outlier

With regard to the fact that the present contribution is devoted to the issue of estimation methods which allow to reveal so-called outlier measurements in the set of measured data, and for which the arithmetic mean was not selected as the centrality parameter, the regression line was deliberately loaded with the only outlier (Fig. 5) in order to track the performance of such methods. In the figure, the experimental values are shown in blue; the line behaviour in deterministic form is shown in red.



Fig. 5 Deterministic form of the regression line loaded by an experimental outlier

Such modified regression line was adjusted first by the method of least squares, and consequently by the simplex method. The results of adjustments of the regression line loaded by an experimental outlier after the adjustment by the LSM and the simplex method are interpreted in Tab. 4 and in Fig. 6.

Tab. 3 Resulting simplex table for the example of the regression line loaded by an experimental outlier

					-	.					-									-					
a	1	0	-1	0	-0.8333	0.8333	0	0	0	0	0	0	0	0	0.8333	-0.8333	0	0	0	0	0	0	0	0	2,0833
b	0	1	0	-1	1.1667	-0.1667	0	0	0	0	0	0	0	0	-1.1667	0.1667	0	0	0	0	0	0	0	0	2,7833
V ₂	0	0	0	0	0.8333	0.1667	-1	0	0	0	0	0	0	0	-0.8333	-0.1667	1	0	0	0	0	0	0	0	0.2167
V3	0	0	0	0	-0.6667	-0.3333	0	1	0	0	0	0	0	0	0.6667	0.3333	0	-1	0	0	0	0	0	0	0.4667
V4	0	0	0	0	-0.5000	-0.5000	0	0	1	0	0	0	0	0	0.5000	0.5000	0	0	-1	0	0	0	0	0	0.8500
V ₅	0	0	0	0	0.3333	0.6667	0	0	0	-1	0	0	0	0	-0.3333	-0.6667	0	0	0	1	0	0	0	0	0.1667
V ₆	0	0	0	0	0.1667	0.8333	0	0	0	0	-1	0	0	0	-0.1667	-0.8333	0	0	0	0	1	0	0	0	0.2833
v ₈	0	0	0	0	0.1667	-1.1667	0	0	0	0	0	1	0	0	-0.1667	1.1667	0	0	0	0	0	-1	0	0	0.3833
V9	0	0	0	0	0.3333	-1.3333	0	0	0	0	0	0	1	0	-0.3333	1.3333	0	0	0	0	0	0	-1	0	2,9667
V10	0	0	0	0	-0.5000	1.5000	0	0	0	0	0	0	0	-1	0.5000	-1.5000	0	0	0	0	0	0	0	1	0.5500
r	0	0	0	0	0.8333	1.1667	2	0	0	2	2	0	0	2	1.1667	0.8333	0	2	2	0	0	2	2	0	-5.8833

 Tab. 4 Comparison of the LSM and the L1 norm on the example of the regression line loaded by an experimental outlier

	TI	neoretical line			LSM		L1
no. m	d	σ_{d}	σ_{d}	8	v	Δ	v
	[mm]	[mm]	[mm]	[mm]	[mm]	[mm]	[mm]
1.	200.00	3.20	3.40	0.20	-0.0436	0.24	0.0000
2.	400.00	3.40	3.80	0.40	0.2594	0.14	0.2167
3.	600.00	4.50	4.20	-0.30	-0.3376	0.04	-0.4667
4.	800.00	5.30	4.60	-0.70	-0.6345	-0.07	-0.8500
5.	1000.00	4.70	5.00	0.30	0.4685	-0.17	0.1667
6.	1200.00	5.00	5.40	0.40	0.6715	-0.27	0.2833
7.	1400.00	5.70	5.80	0.10	0.4745	-0.37	0.0000
8.	1600.00	6.50	6.20	-0.30	0.1776	-0.48	-0.3833
9.	1800.00	9.50	6.60	-2.90	-2.3194	-0.58	-2.9667
10.	2000.00	6.40	7.00	0.60	1.2836	-0.68	0.5500

Parameters of regression line:

Deterministic form of line Parameters of line estimated by LSM Parameters of line estimated by L1 y= 3.0[mm]+ 2.0*ppm*d y= 2.7[mm]+ 2.5*ppm*d y= 2.1[mm]+ 2.8*ppm*d



Fig. 6 10 Graphic interpretation of the LSM and the L1 norm (simplex method) on the example of the regression line loaded by an experimental outlier

From the results of the adjustment through the LSM, it is evident that in this case the LSM found an outlier measurement at a measured length of 1800 m and just this measurement was assigned the greatest value of correction after the adjustment. Comparing the two estimation procedures, we concluded that both methods

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arrive at a mutually similar results and the simplex method found an outlier measurement at a measured length of 1800 m.

4.3 Adjustment of a geodetic network loaded by a normal distribution of measurements

Characteristics of the presented estimation methods will also be investigated on the example of a geodetic network. This is a simulated geodetic network where the point No. 6 is the point being determined whose position is defined by triangulation (angular) measurements (Fig. 7). Such a network model was proposed on the grounds that the geodetic practice is often encountered with the task when it is necessary to determine the location of an inaccessible point which can be done just through triangulation measurements. The geodetic network will be processed as a binding network by both network estimative procedures, first by a normal distribution of measurements, and consequently after it is loaded by outlier experimental values.



Fig. 7 Structure of the geodetic network loaded by a normal distribution of measurements

Tab. 1	Results	of adjust	ment of the	geodetic network	loaded by a not	rmal distribution	through the LSM
--------	---------	-----------	-------------	------------------	-----------------	-------------------	-----------------

L-S-P	l~	1	۱^	v~	v	vC^	р	T.Baarda	T.Pope	s(v)	s(l^)	r *
angle	[g]	[g]	[g]	[cc]	[cc]	[cc]		- B -	-P-	[cc]		
2-1-6	31.04550	31.04500	31.04559	4.95	5.94	5.94	1.0000	1.58	1.43	3.76	4.15	0.88
6-1-5	86.04331	86.04290	86.04321	4.08	3.09	3.09	1.0000	0.82	0.74	3.76	4.15	0.88
3-2-6	57.35031	57.35090	57.35053	-5.90	-3.72	-3.72	1.0000	1.17	1.06	3.18	3.51	0.63
6-2-1	82.30387	82.30390	82.30366	-0.26	-2.45	-2.45	1.0000	0.77	0.70	3.18	3.51	0.63
4-3-6	46.63242	46.63190	46.63229	5.21	3.95	3.95	1.0000	1.18	1.07	3.33	3.68	0.69
6-3-2	43.61620	43.61630	43.61633	-0.98	0.29	0.29	1.0000	0.09	0.08	3.33	3.68	0.69
5-4-6	94.62331	94.62290	94.62322	4.11	3.15	3.15	1.0000	0.84	0.76	3.74	4.13	0.88
6-4-3	32.88348	32.88290	32.88358	5.81	6.76	6.76	1.0000	1.81	1.64	3.74	4.13	0.88
1-5-6	71.37301	71.37290	71.37303	1.07	1.28	1.28	1.0000	0.33	0.30	3.83	4.23	0.92
6-5-4	54.12859	54.12900	54.12857	-4.08	-4.29	-4.29	1.0000	1.12	1.01	3.83	4.23	0.92

Legend:

Significance level alpha	= 0.050	B - Baarda Data snooping (N(0,1))	= 2.800
Basic standard deviation of angle mesur.	= 4.000 [cc]	Number of critical measure. (Baarda data-snooping)	= 0
Estimate of the accuracy of angle m. (MINQUE)	= 4.417 [cc]	P - Pope Tau-test (Tau(r, 1-alfa0/2)	= 2.361
Posterior standard deviation	= 4.417 [cc]	Number of critical measure. Pope Tau-test)	= 0
Critical limit s0_posterior	= 5.569 [cc]	Adjustment efficiency	= 0.800
s0_poster^2/s0_aprior^2	= 1.219	Redundancy	= 8.000

The results of adjustment of thus proposed geodetic network by the method of least squares are presented in

Tab. 1. In the case of the network loaded by a normal distribution of measurements, the standard deviation of the measured angle takes a value of 4.000 cc. The accuracy of the angular measurement 4.417 cc was estimated by the method MINQUE [10],[12]. The fact whether the set of measured data was

infiltrated by an outlier measurement was studied using two statistical tests Baarda Data snooping test and the Pope Tau test. From the results of processing (

Tab. 1), it is clear that not only the LSM did not find outlier measurements, but none of the measurements exceeded the critical value of both test statistics. Due to the investigation of the properties of alternative estimation methods, the network was adjusted again by the simplex method (

Tab. 2).

	The stand simplex were in processing the geodetic network folded by a normal distribution																								
dX ₆ ^	1	0	-1	0	0.032	-0.085	0	0	0	0	0	0	0	0	-0.032	0.085	0	0	0	0	0	0	0	0	0.1078
dY ₆ ^	0	1	0	-1	-0.069	-0.042	0	0	0	0	0	0	0	0	0.069	0.042	0	0	0	0	0	0	0	0	-0.4497
V _{u(2,1,6)}	0	0	0	0	0.109	0.591	-1	0	0	0	0	0	0	0	-0.109	-0.591	1	0	0	0	0	0	0	0	6.1698
V _{u(6,1,5)}	0	0	0	0	-0.109	-0.591	0	-1	0	0	0	0	0	0	0.109	0.591	0	1	0	0	0	0	0	0	2.8602
V _{u(6,2,1)}	0	0	0	0	1	0	0	0	1	0	0	0	0	0	-1	0	0	0	-1	0	0	0	0	0	6.1600
Vu(4,3,6)	0	0	0	0	0	-1	0	0	0	-1	0	0	0	0	0	1	0	0	0	1	0	0	0	0	4.2300
V _{u(5,4,6)}	0	0	0	0	-0.566	0.223	0	0	0	0	-1	0	0	0	0.566	-0.223	0	0	0	0	1	0	0	0	0.9912
V _{u(6,4,3)}	0	0	0	0	0.566	-0.223	0	0	0	0	0	-1	0	0	-0.566	0.223	0	0	0	0	0	1	0	0	8.9288
Vu(1,5,6)	0	0	0	0	-0.193	0.499	0	0	0	0	0	0	-1	0	0.193	-0.499	0	0	0	0	0	0	1	0	0.4197
Vu(6,5,4)	0	0	0	0	-0.193	0.499	0	0	0	0	0	0	0	1	0.193	-0.499	0	0	0	0	0	0	0	-1	3.4297
r	0	0	0	0	0.386	1.001	2	2	0	2	2	2	2	0	1.614	0.999	0	0	2	0	0	0	0	2	-33.1894

Tab. 2 Rresulting simplex table in processing the geodetic network loaded by a normal distribution

Tab. 3 Estimates of the coordinates of the point No. 6 from measurements loaded by a normal distribution

	X°	Y°	dX^	dY^	X^	Y^
Method	[m]	[m]	[mm]	[mm]	[m]	[m]
LSM	1167044 502	429102 961	-0.0374	-0.2039	1167044.50196	438102.86080
L1	1107044.302	438102.801	0.1078	-0.4497	1167044.50211	438102.86055

Tab. 4 Comparing corrections in the geodetic network non-loaded by outlier measurements

Angle	l~ Theoretical value of angle	l Measured value of angle	v~ Actual measurement error	v LSM	v Simplex
L-S-P	[g]	[g]	[cc]	[cc]	[cc]
2-1-6	31.04550	31.04500	4.95	5.94	6.17
6-1-5	86.04331	86.04290	4.08	3.09	2.86
3-2-6	57.35031	57.35090	-5.90	-3.72	0.00
6-2-1	82.30387	82.30390	-0.26	-2.45	-6.16
4-3-6	46.63242	46.63190	5.21	3.95	4.23
6-3-2	43.61620	43.61630	-0.98	0.29	0.00
5-4-6	94.62331	94.62290	4.11	3.15	0.99
6-4-3	32.88348	32.88290	5.81	6.76	8.93
1-5-6	71.37301	71.37290	1.07	1.28	0.42
6-5-4	54.12859	54.12900	-4.08	-4.29	-3.43

The graphical interpretation of the results of adjustment (Fig. 8) is presented through the confidence ellipses constructed at 99% and 95% probability.



Fig. 8 Graphical interpretation of the estimation methods for adjusting the geodetic network loaded by a normal distribution of measurements

4.4 The adjustment of the geodetic network loaded by a normal distribution of measurements

To investigate the properties of alternative estimation methods enabling to detect outlier measurements that can infiltrate such a set of measured data, e.g. by a supraliminal influence of the physical environment, two measured angles were loaded in the proposed geodetic network prior to the processing (Fig. 9), once by five times the mean error of the measured angle, once by six times the estimated error. Such proposed geodetic network was again adjusted as the binding network by the (MNS) (

Tab. 5) and by the simplex method (

Tab. 7).



Fig. 9 Structure of the geodetic network loaded by two outlier measurements

L-S-P	l~	1	1^	v~	v	vC^	р	T.Baarda	T.Pope	s(v)	s(l^)	r *	f
angle	[g]	[g]	[g]	[cc]	[cc]	[cc]		- B -	-P-	[cc]			
2-1-6	31.04550	31.04790	31.04581	-24.00	-20.86	-20.86	1.0000	5.55 B	2.02	3.76	10.33	0.88	66.0
6-1-5	86.04331	86.04290	86.04299	4.08	0.94	0.94	1.0000	0.25	0.09	3.76	10.33	0.88	66.0
3-2-6	57.35031	57.35090	57.34998	-5.90	-9.24	-9.24	1.0000	2.91 B	1.06	3.18	8.72	0.63	39.2
6-2-1	82.30387	82.30587	82.30421	-20.00	-16.66	-16.66	1.0000	5.25 B	1.91	3.18	8.72	0.63	39.2
4-3-6	46.63242	46.63190	46.63183	5.21	-0.71	-0.71	1.0000	0.21	0.08	3.33	9.14	0.69	44.6
6-3-2	43.61620	43.61630	43.61679	-0.98	4.94	4.94	1.0000	1.48	0.54	3.33	9.14	0.69	44.6
5-4-6	94.62331	94.62290	94.62363	4.11	7.32	7.32	1.0000	1.95	0.71	3.74	10.28	0.88	64.7
6-4-3	32.88348	32.88290	32.88316	5.81	2.60	2.60	1.0000	0.70	0.25	3.74	10.28	0.88	64.7
1-5-6	71.37301	71.37290	71.37337	1.07	4.67	4.67	1.0000	1.22	0.44	3.83	10.52	0.92	71.1
6-5-4	54.12859	54.12900	54.12823	-4.08	-7.68	-7.68	1.0000	2.01	0.73	3.83	10.52	0.92	71.1
Signific Basic st	ance level al andard devia	pha tion of angul	ar meas.	= ().050 4.000 [cc]		B - Baa Numbe	rda Data snoo r of critical me	ping (N(0 eas. (Baard),1)) a)	=	2.800 3	

Tab. 5 Results of adjustment of the geodetic network loaded by outlier measurements through the LSM

Significance level alpha	=	0.050
Basic standard deviation of angular meas.	=	4.000 [cc]
Estimate of precision of angular meas. (MINQUE)	=	10.984 [cc]
Posterior standard deviation	=	10.984 [cc]
Critical limit s0_posterior	=	5.569 [cc]
s0_poster^2/s0_aprior^2	=	7.541
Crit. ratio s0_poster^2/s0_aprior^2	=	1.938

B - Baarda Data snooping (N(0,1))
Number of critical meas. (Baarda)
P - Pope Tau-test (Tau(r, 1-alfa0/2)
Number of critical meas. (Pope Tau-test)
Redundancy

Results of the adjustment of the geodetic network show that the LSM found two outliers just on the measured angles loaded by multiples of the median error of the measured angle. Infiltrating outlier measurements to the set of the processed data, however, was also studied through the Baarda Snooping test and the Pope test. From

= 0

= 2.361

= 0.800

Tab. 5, it is evident that the Baard test found three critical measurements, while the Pope test did not find any critical test measurement. For this reason, the simplex method (

Tab. 7) was used for the processing of the geodetic network.

Tuble 0 Estimates of the point 1(0, 0 coordinates from measurements founded by a normal distribution													
	X°	Y°	dX^	dY^	Χ^	Y^							
Method	[m]	[m]	[mm]	[mm]	[m]	[m]							
LSM	1167044 502	438102 861	-0.6136	-0.0144	1167044.50139	438102.86099							
L1	1107044.502	430102.001	-0.2529	-0.6252	1167044.50175	438102.86037							

Tab. 6 Estimates of the point No. 6 coordinates from measurements loaded by a normal distribution

Tab. 7 Resulting simplex table in processing the geodetic network loaded by outlier measurements

dX6^	1	0	-1	0	0.032	0.085	0	0	0	0	0	0	0	0	-0.032	-0.085	0	0	0	0	0	0	0	0	-0.2529
dY6^	0	1	0	-1	-0.069	0.042	0	0	0	0	0	0	0	0	0.069	-0.042	0	0	0	0	0	0	0	0	-0.6252
v _{u(2,1,6)}	0	0	0	0	-0.109	0.591	1	0	0	0	0	0	0	0	0.109	-0.591	-1	0	0	0	0	0	0	0	20.3326
v _{u(6,1,5)}	0	0	0	0	-0.109	0.591	0	-1	0	0	0	0	0	0	0.109	-0.591	0	1	0	0	0	0	0	0	0.3626
$v_{u(6,2,1)}$	0	0	0	0	1	0	0	0	1	0	0	0	0	0	-1	0	0	0	-1	0	0	0	0	0	25.8600
v _{u(6,3,2)}	0	0	0	0	0	-1	0	0	0	-1	0	0	0	0	0	1	0	0	0	1	0	0	0	0	4.2300
v _{u(5,4,6)}	0	0	0	0	-0.566	-0.223	0	0	0	0	-1	0	0	0	0.566	0.223	0	0	0	0	1	0	0	0	1.9341
v _{u(6,4,3)}	0	0	0	0	0.566	0.223	0	0	0	0	0	-1	0	0	-0.566	-0.223	0	0	0	0	0	1	0	0	7.9859
$v_{u(1,5,6)}$	0	0	0	0	-0.193	-0.499	0	0	0	0	0	0	-1	0	0.193	0.499	0	0	0	0	0	0	1	0	2.5317
v _{u(6,5,4)}	0	0	0	0	-0.193	-0.499	0	0	0	0	0	0	0	1	0.193	0.499	0	0	0	0	0	0	0	-1	5.5417
r	0	0	0	0	0.604	1.818	0	2	0	2	2	2	2	0	1.396	0.182	2	0	2	0	0	0	0	2	-68.7786

The resulting simplex table shows that in the processing of geodetic network using this method, the optimal solution was found, since all elements of the vector r are positive and the table of resulting corrections (

Tab. 8) points out to the fact that this method identified two outlier measurement at the points where the angles are loaded by multiples of the standard deviation of the measured angle. The graphical interpretation of the results obtained is shown through the confidence ellipses constructed at 95% and 99% probability (Fig. 10).

Tab.	8 C	Comparing	the	corrections	in tł	he g	geodetic	network	loaded	by	outlier	measurem	ents

1		Ū.			
Angle	l~ Theoretical value of angle	l Measured value of angle	v~ Actual measurement error	v LSM	v Simplex
L-S-P	[g]	[g]	[cc]	[cc]	[cc]
2-1-6	31.04550	31.04790	-24.00	-20.86	-20.33
6-1-5	86.04331	86.04290	4.08	0.94	0.36
3-2-6	57.35031	57.35090	-5.90	-9.24	0.00
6-2-1	82.30387	82.30587	-20.00	-16.66	-25.86
4-3-6	46.63242	46.63190	5.21	-0.71	0.00
6-3-2	43.61620	43.61630	-0.98	4.94	4.23
5-4-6	94.62331	94.62290	4.11	7.32	1.93
6-4-3	32.88348	32.88290	5.81	2.60	7.99
1-5-6	71.37301	71.37290	1.07	4.67	2.53
6-5-4	54.12859	54.12900	-4.08	-7.68	-5.54



Fig. 10 Graphical interpretation of the estimation methods for adjusting the geodetic network loaded by outlier measurements

5 CONCLUSIONS

The estimation methods that were used and applied to the example of a regression line and a geodetic network showed the mutual tightness of the achievements. In the present contribution, simple but all the more illustrating examples demonstrating the positive attributes of alternative estimation methods were deliberately chosen. Among the many such methods published in foreign literature [8], the simplex method was presented in this paper which also allows to solve the problem of infiltration of outlier measurements to the set of processed data. The minimization problem of the L1-norm is normally solved using a linear programming tabular form; however, the paper presents a simpler and more efficient way to solve a linear programming problem using matrix relations.

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RESUMÉ

Záverom si dovoľujeme konštatovať, že použitie alternatívnych odhadovacích metód má nesporne svoje opodstatnenie, pretože geodetické merania sa nezriedka realizujú v náročných a neštandardných podmienkach, ktoré môžu byť príčinou skutočnosti, že do súboru meraných dát preniknú odľahlé merania. Z opakovaných meraní na stanovisku a z obmedzených doplnkových fyzikálnych meraní teploty, tlaku a vlhkosti má geodet len obmedzenú možnosť posúdiť, či a do akej miery sa do súborov meraných geometrických veličín infiltroval vplyv predovšetkým tepelného poľa, prípadne iných rušivých fenoménov, o ktorom môže prijať závery zväčša až na základe štatistického spracovania po návrate z terénu. Pokiaľ sa jedná o jedno meranie, je identifikovateľné napr. štatistickými testami, v prípade, že súbor je kontaminovaný viacerými odľahlými meraniami, štatistické testy nemusia byť úspešné. V tomto prípade alternatívne odhadovacie metódy, kde nesporne patria aj metódy lineárneho programovania dokážu identifikovať túto množinu a potlačiť ich vplyv na výsledky vyrovnania.